INDEX OF SHEETS

01 COVER & INDEX OF SHEETS

O2 GENERAL NOTES & SUMMARY OF QUANTITIES

03 - 05 TRAFFIC CONTROL PLANS
06 REMOVAL PLAN AND DETAILS

07 DECK PLAN
08 DETAILS

09 – 11 PREFORMED JOINT SEAL 12 BAR SPLICER DETAILS

13 TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION

HIGHWAY STANDARDS

000001–06 STANDARD SYMBOLS, ABBREVIATIONS, AND PATTERNS

001002-02 AREAS OF REINFORCEMENT BARS

001006 DECIMAL OF AN INCH AND OF A FOOT

606001-01 CONCRETE CURB TYPE B AND COMBINATION CONCRETE CURB AND GUTTER

701321-07 LANE CLOSURE, 2L, 2W, BRIDGE REPAIR WITH BARRIER

704001–08 TEMPORARY CONCRETE BARRIER

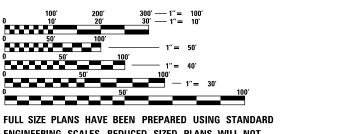
701801–06 SIDEWALK, CORNER OR CROSSWALK CLOSURE

701901–07 TRAFFIC CONTROL DEVICES

DESIGN DESIGNATION:

OREGON STREET: MAJOR COLLECTOR (URBAN)

2014 ADT = 3,200



FULL SIZE PLANS HAVE BEEN PREPARED USING STANDARD ENGINEERING SCALES. REDUCED SIZED PLANS WILL NOT CONFORM TO STANDARD SCALES. IN MAKING MEASUREMENTS ON REDUCED PLANS, THE ABOVE SCALES MAY BE USED.

J.U.L.I.E.

JOINT UTILITY LOCATION INFORMATION FOR EXCAVATION

1-800-892-0123

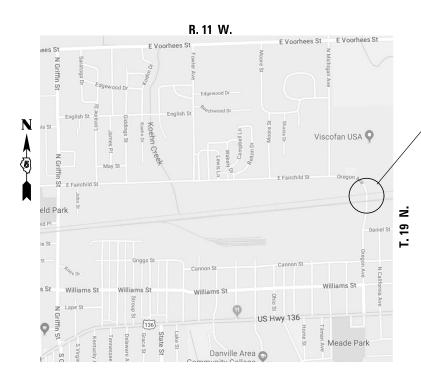
STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION

PROPOSED HIGHWAY PLANS

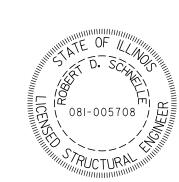
OREGON STREET OVER THE CSX TRANSPORTATION SECTION 16-00353-00-BR

BRIDGE JOINT AND APPROACH
PAVEMENT REPAIRS
STRUCTURE NUMBER 092–6017
CITY OF DANVILLE

FUNDING MFT



LOCATION MAP



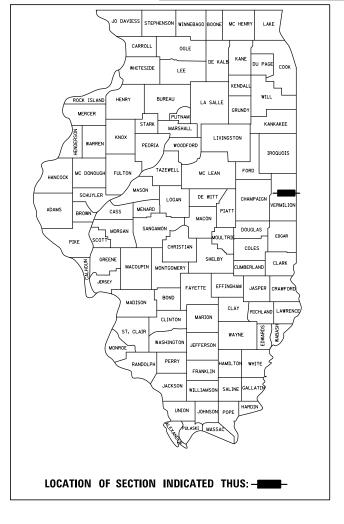
EXISTING S.N. 092-6017 OREGON STREET OVER

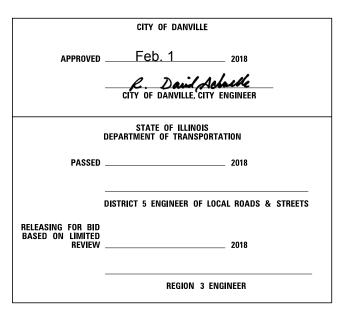
CSX TRANSPORTATION
PROPOSED BRIDGE JOINT

REPAIRS

f. Daid Adulle

1/3/20





CITY ENGINEER, ILLINOIS STRUCTURAL NO. 081–005708
LICENSE EXPIRES: 11–30–2018

GENERAL NOTES

- All construction shall be done in accordance with the State of Illinois
 "Standard Specifications for Road And Bridge Construction adopted April 1, 2016",
 these plans, and the special provisions included in the contract documents
- The revision numbers of the standards listed in the plans are to be used for constructing of this section.
- The location of existing utilities are not shown. It shall be the contractor's responsibility to ascertain their exact location from the individual utility companies and by field inspection.
- 4. Any time the concrete barriers are not in the proper position, flaggers shall be in place to control traffic and the temporary traffic signals shall be turned off or covered.
- 5. Commitments: None as of >date
- 6. Reinforcement bars designated (E) shall be epoxy coated.
- 7. Prior to pouring the new concrete deck, all heavy or loose rust, loose mill scale, and other loose or potentially detrimental foreign material shall be removed from the surfaces in contact with concrete. Tightly adhered paint may remain unless otherwise noted. Removal shall be accomplished by methods that will not damage the steel and the cost will be included in the pay item covering removal of the existing concrete.

As directed by the Engineer, existing construction accessories welded to the top flange of beams and girders shall be removed. The weld areas shall be ground flush and inspected for cracks using magnetic particle testing (MT) or dye penetrant testing (PT) by qualified personnel approved by the Engineer.

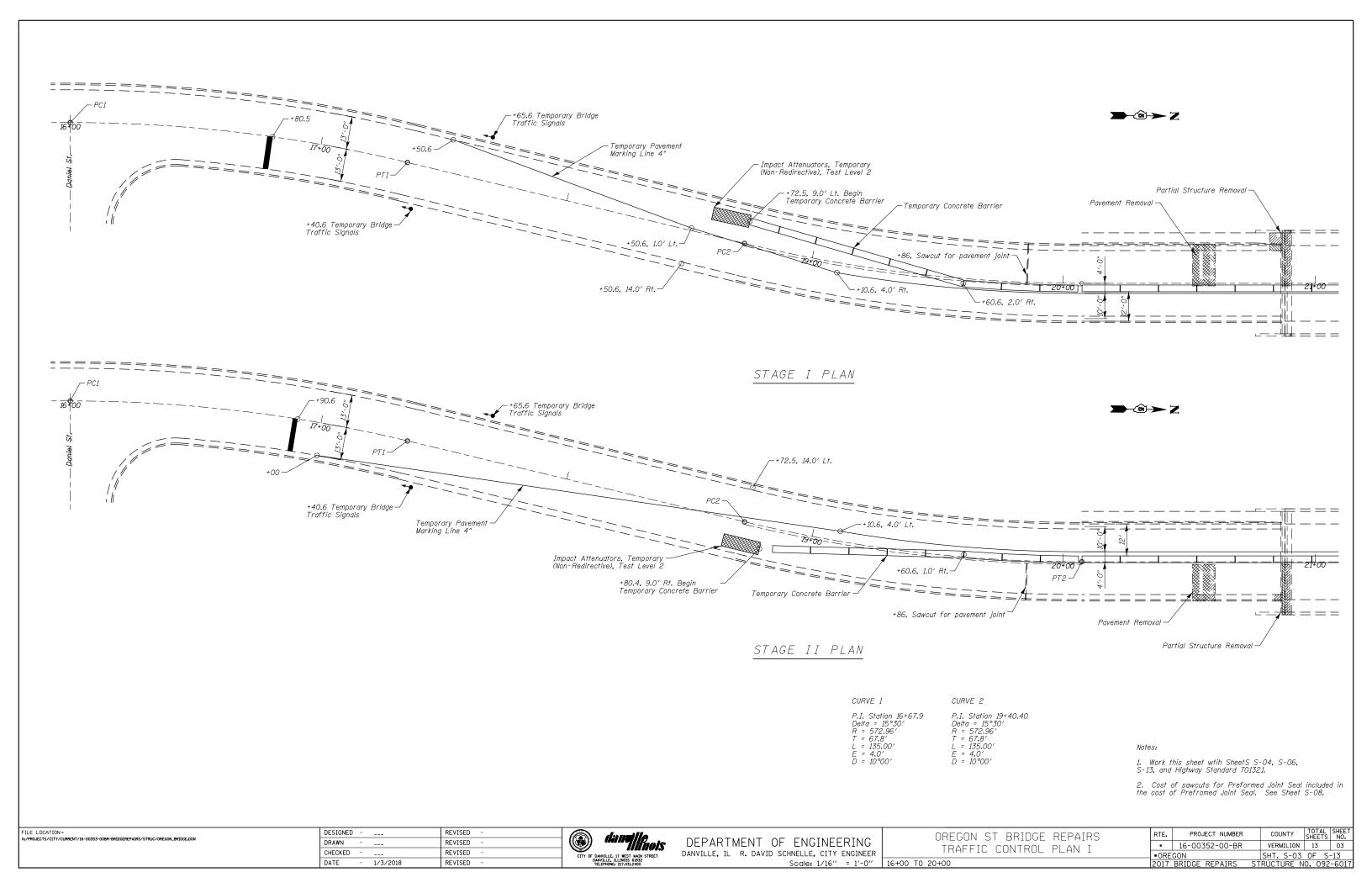
Any cracks that cannot be removed by grinding 1#4 inch deep shall be identified and reported to the Bureau of Bridges and Structures for further disposition. The cost of removing welded accessories, grinding and inspecting weld areas and grinding cracks will be paid for according to Article 109.04 of the Standard Specifications.

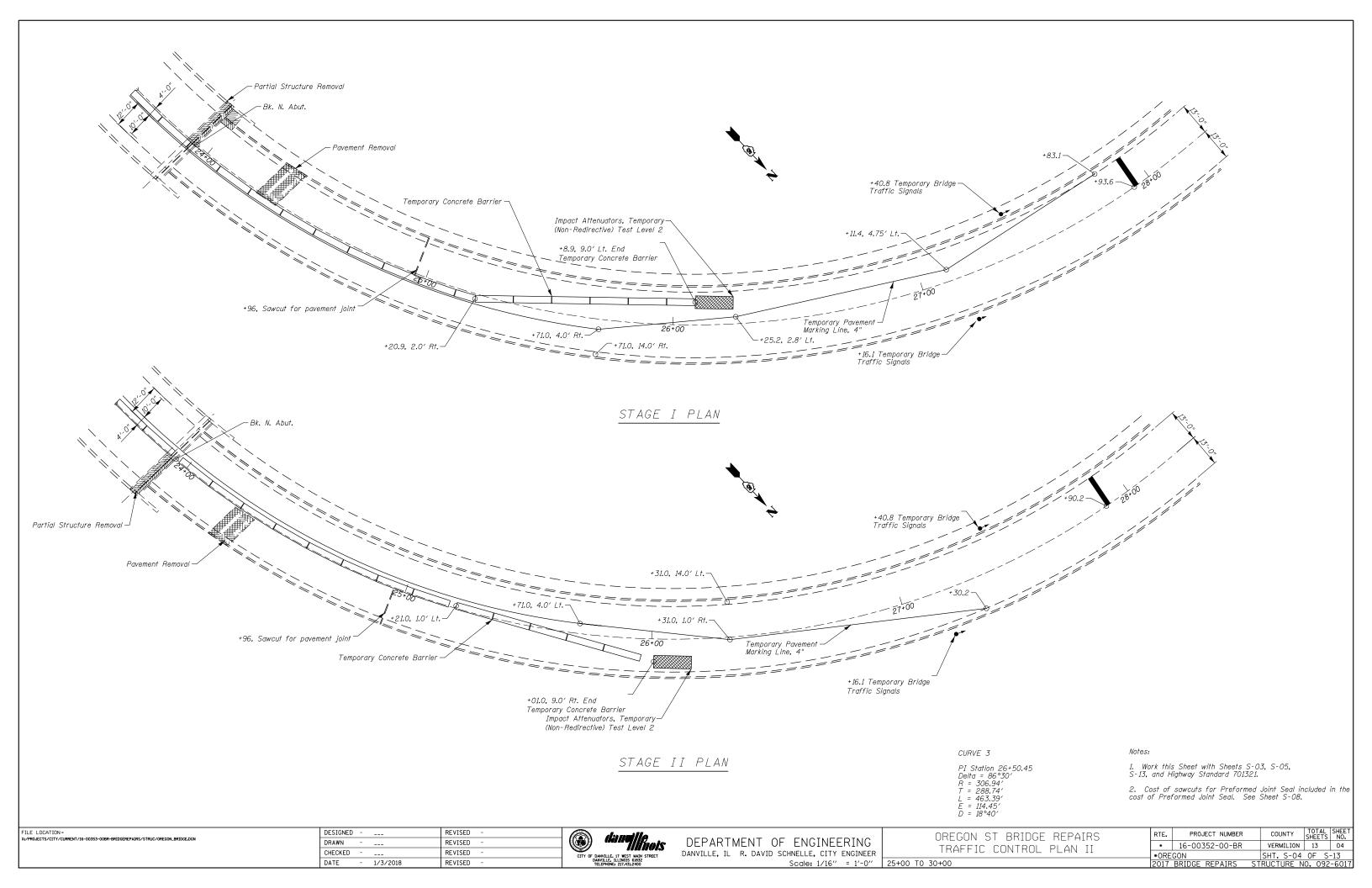
- 8. Plan dimensions and details relative to existing plans are subject to nominal construction variations. The Contractor shall field verify existing dimensions and details affecting new construction and make necessary approved adjustments prior to construction or ordering of materials. Such variations shall not be cause for additional compensation for a change in scope of the work, however, the Contractor will be paid for the quantity actually furnished at the unit price bid for the work.
- Existing reinforcement shall be cleaned and incorporated into the new construction. Cost included with Concrete Removal.
- 10. Combination concrete curb and gutter abutting existing pavement shall be constructed and paid for according to Section 606 of the Standard Specifications.

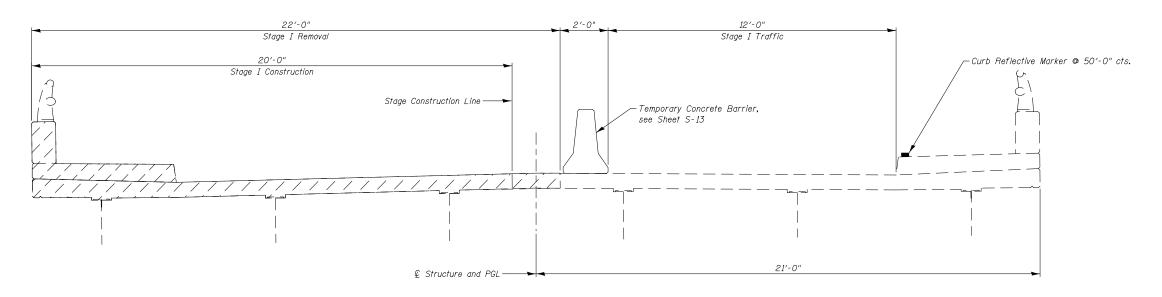
SUMMARY OF QUANTITIES

PAY ITEM NO.	DESCRIPTION	UNIT	TOTAL
44002100	CONTINUOUSLY REINFORCED CONCRETE PAVEMENT REMOVAL		62.9
50102400	CONCRETE REMOVAL	CU YD	15.1
50300255	CONCRETE SUPERSTRUCTURE	CU YD	14.9
50300300	PROTECTIVE COAT	SQ YD	50
50800206	REINFORCEMENT BARS, EPOXY COATED	POUND	6110
50800515	BAR SPLICERS	EACH	100
52000050	PREFORMED JOINT SEAL 4"	F00T	65
52000110	PREFORMED JOINT STRIP SEAL	F00T	80
67100100	MOBILIZATION	L SUM	1
70100405	TRAFFIC CONTROL AND PROTECTION STANDARD 701321	EACH	1
70102640	TRAFFIC CONTROL AND PROTECTION STANDARD 701801	L SUM	1
70106500	TEMPORARY BRIDGE TRAFFIC SIGNALS	EACH	1
70300220	TEMPORARY PAVEMENT MARKING LINE 4"	F00T	2083
70400100	TEMPORARY CONCRETE BARRIER	F00T	737
70400200	RELOCATE TEMPORARY CONCRETE BARRIER	F00T	723
42400200	PORTLAND CEMENT CONCRETE SIDEWALK 5 INCH	SQ FT	50
44000500	COMBINATION CURB AND GUTTER REMOVAL	FT	13
44000600	SIDEWALK REMOVAL	SQ FT	50
60605100	COMBINATION CONCRETE CURB AND GUTTER TYPE B6.24 (AEP)	FT	13
70600240	IMPACT ATTENUATORS, TEMPORARY (NON- REDIRECTIVE), TEST LEVEL 2	EACH	2
70600340	IMPACT ATTENUATORS, RELOCATE (NON- REDIRECTIVE), TEST LEVEL 2	EACH	2
78200020	CURB REFLECTORS	EACH	14
X7030005	TEMPORARY PAVEMENT MARKING REMOVAL	SQ FT	740

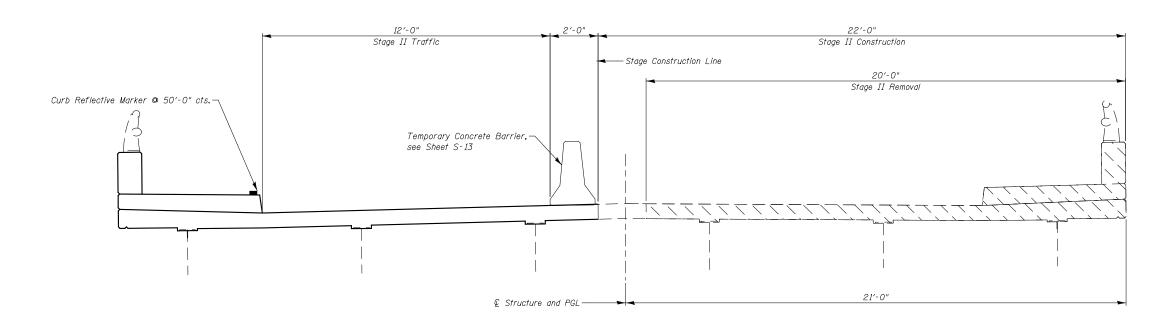




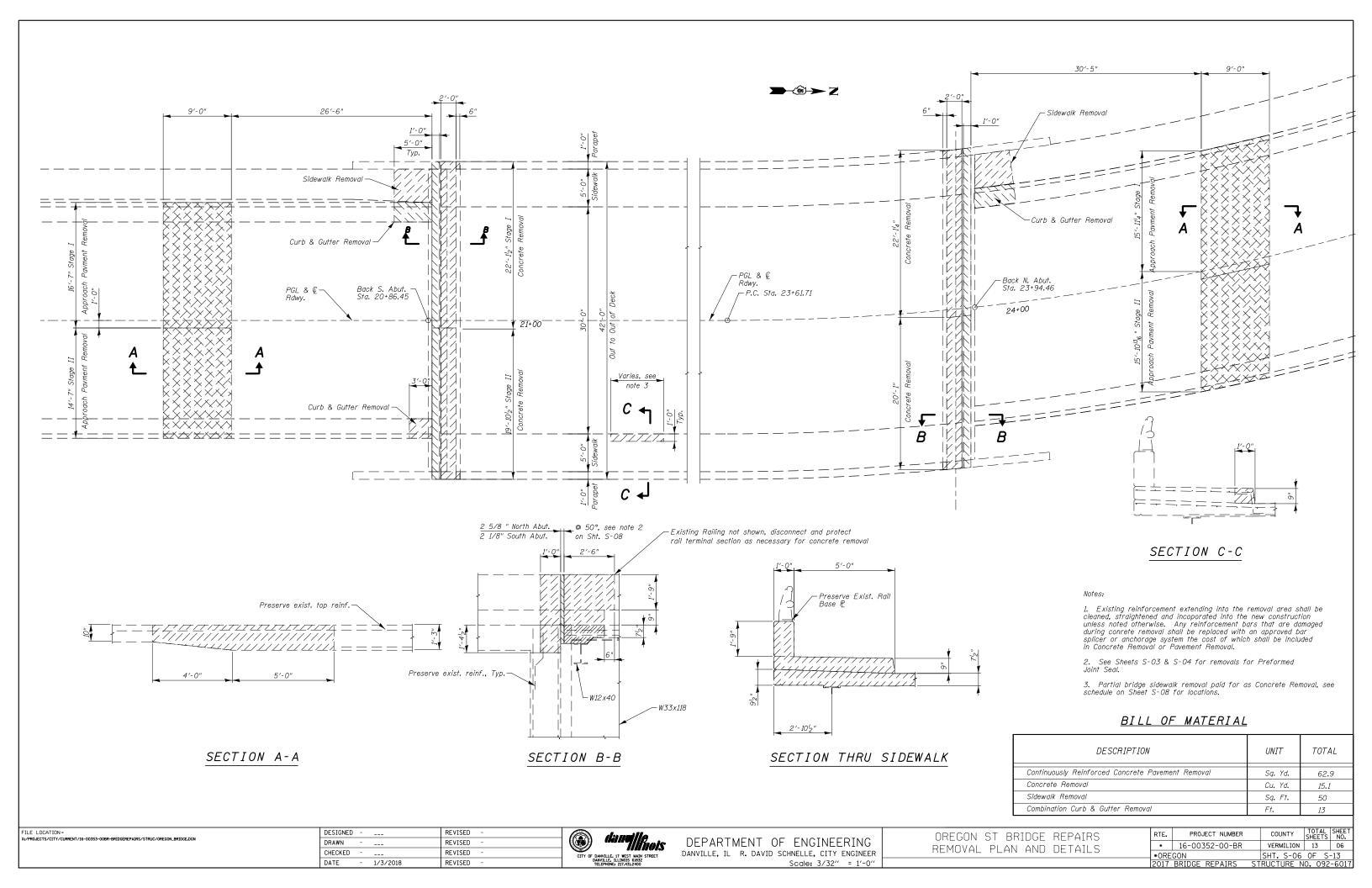


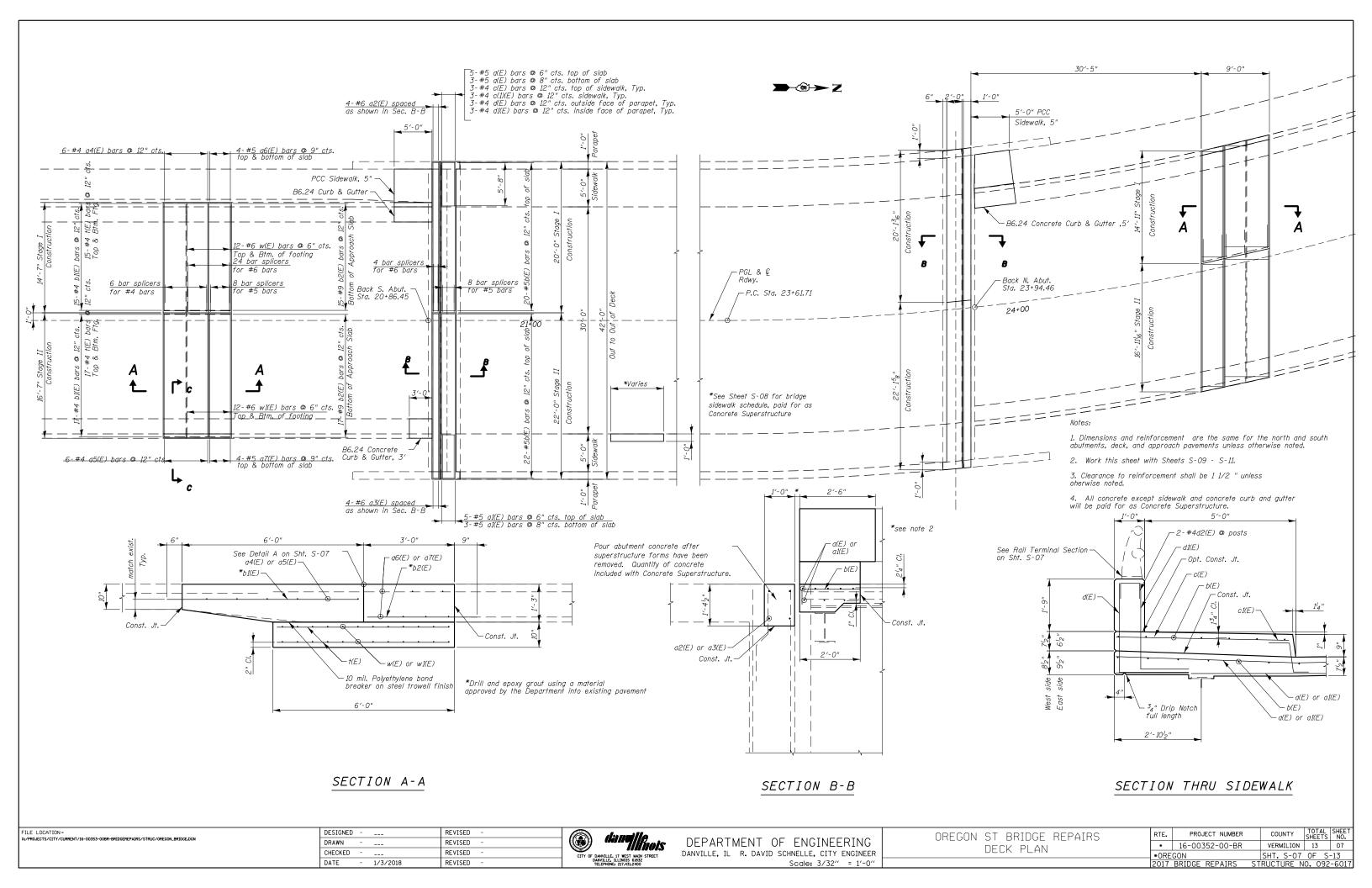


STAGE I CONSTRUCTION



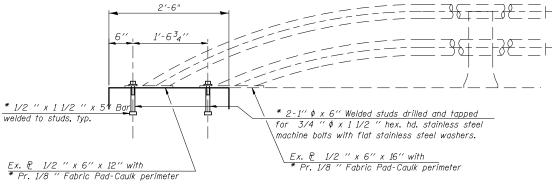
STAGE II CONSTRUCTION



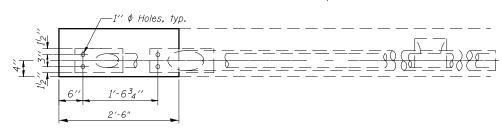


Notes:

- 1. The joint opening shall be determined per Article 520.04. The minimum dimension shall be 1 1/2" for installation purposes.
- 2. Preformed Joint Seal installed in existing pavement will be measured and paid for according to Article 520 of the Standard Specifications. Preformed Joint Seal installed in new concrete shall be included in the cost of Concrete Superstructure.

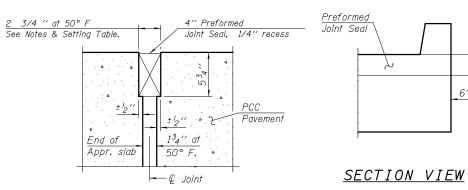


* Cost included with Concrete Superstructure

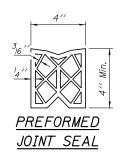


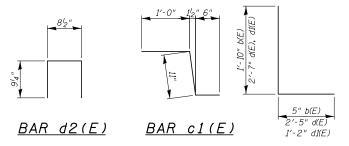
FILE LOCATION =

RAIL TERMINAL SECTION



DETAIL A





BARS b(E), d(E) & d1(E)

BRIDGE SIDEWALK SCHEDULE

Start Station	End Station	Location	*Length
22+19	22+33	Lt.	14'-0"
21+11	21+18	Rt.	7′-0"
21+68	21+72	Rt.	4'-0"
21+93	22+00	Rt.	7′-0"
22+17	22+30	Rt.	13′-0"
22+55	22+61	Rt.	6′-0"
22+71	22+81	Rt.	10'-0"
23+00	23+37	Rt.	37′-0"

^{*}The Engineer shall field verify locations and lengths of repairs. See Section C-C on Sheet S-06 for removal limits

JOINT SETTING TABLE

Air Temp.(F)	North Abut. Joint (inch)	South Abut. Joint (inch)	Approach Pvmt. Joints (inch)
15	2.25	1.75	3. 75
20	2.25	<i>1</i> .50	3.75
25	2.25	1.50	3.50
30	2.00	1.50	3.50
35	2.00	1.50	3. 25
40	2.00	1.50	3. 25
45	1.75	1.50	3.00
50	1.75	1.50	2.75
55	1.75	1.50	2.75
60	1.75	1.50	2.50
65	1.50	1.50	2.50
70	1.50	1.50	2.25
75	1.50	<i>1</i> .50	2.25
80	1 . 50	1 . 50	2.00
85	1 . 50	1.50	2.00
90	1.50	1.50	1.75
95	1.50	<i>1</i> ,50	1.50
100	1.50	1. 50	1.50

BILL OF MATERIAL

Bar	No.	Size	Length	Shape
a(E)	16	#5	19'-9"	
a1(E)	16	#5	21'-9"	
a2(E)	8	#6	19'-9"	
a3(E)	8	#6	21'-9"	
a4(E)	12	#4	14'-8"	
a5(E)	12	#4	16′-8"	
a6(E)	16	#5	14'-3"	
a7(E)	16	#5	16′-3"	
b(E)	112	#5	2'-3"	
b1(E)	64	#4	6'-4"	
b2(E)	64	#9	3′-7"	
c(E)	12	#4	5′-9"	
c1(E)	12	#4	2′-5"	
d(E)	12	#4	5′-0"	
d1(E)	12	#4	3′-9"	
d2(E)	16	#4	2'-3"	
t(E)	128	#4	5′-9"	
w(E)	48	#6	14'-3"	
w1(E)	48	#6	16′-3"	
Concrete Supe	erstructure		Cu. Yd.	14.9
Protective Coat			Sq. Yd.	50
Reinforcement Bars, Epoxy Coated			Pound	6110
Bar Splicers			Each	100
Preformed Joi	nt Seal 4"	Foot	65	
PCC Sidewalk	5"	Sq. Ft.	50	
*Combination	Concrete Curb	Ft.	13	

*See General Note 10 on Sheet S-02

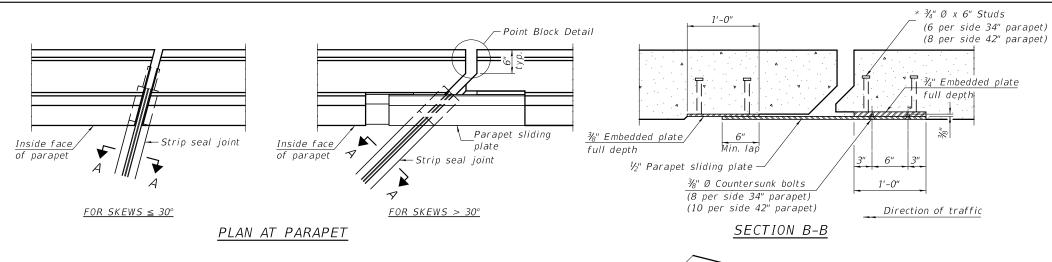
DESIGNED - ___ REVISED DRAWN REVISED CHECKED REVISED DATE - 1/3/2018 REVISED



DEPARTMENT OF ENGINEERING DANVILLE, IL R. DAVID SCHNELLE, CITY ENGINEER Scale: N.A. = 1'-0"

OREGON ST BRIDGE REPAIRS DETAILS

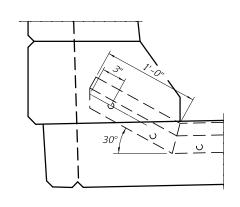
PROJECT NUMBER COUNTY • 16-00352-00-BR VERMILION 13 08 SHT. S-08 OF S-13 STRUCTURE NO. 092-601



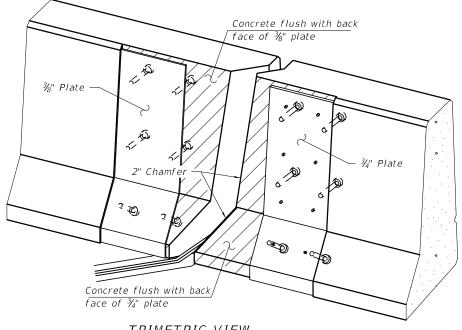
Detail A B Top of locking edge rail Inside Face of Parapet Top of locking edge rail Top of deck Top of deck

ELEVATION AT PARAPET

(Skews > 30° shown. Skews = 30° similar except as shown in plan view.)



DETAIL A



TRIMETRIC VIEW
(Showing embedded plates only)

3/4" N. Abut.

Locking edge rail 1 3/4" N. Abut. 1 1/2" S. Abut. © 50° F Top of concrete Strip seal * 5%" Ø angled 2 5/8" N. Abut. 2 1/8" S. Abut. 2 1/8" S. Abut. 50° F

SHOWING ROLLED RAIL JOINT

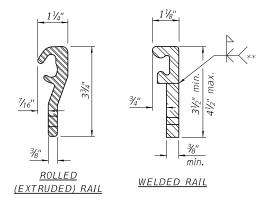
* 1/8" Ø x 6" studs @ 6" cts. (alternate angled/bent studs with horizontal studs)

3/8" \$\phi\$ threaded rods in 7/6" \$\phi\$ holes at \$\pm 4'-0"\$ cts. for holding the proper joint opening based on the temperature during the deck pour. Place to miss studs. All rods shall be burned, or sawed off flush with the plates after concrete is set.

SHOWING WELDED RAIL JOINT

SECTION A-A

* Granular or solid flux filled headed studs conforming to Article 1006.32 of the Std. Specs., automatically end welded.



LOCKING EDGE RAILS

** Back gouge not required if complete joint penetration is verified by mock-up.

Notes

The strip seal shall be made continuous and shall have a minimum thickness of $\frac{1}{4}$ ". The configuration of the strip seal shall match the configuration of the locking edge rails. Open or "webbed" strip seal gland configurations are not permitted. The gland shall be sized for a maximum rated movement of 4 inches.

The locking edge rails depicted are configured for typical applications and are conceptual only. The actual configuration of the locking edge rails and matching strip seal may vary from manufacturer to manufacturer provided they fit the application and meet the minimum anchorage shown. Flanged edge rails, however, will not be allowed. Locking edge rails may exceed the 4½" maximum depth provided the anchorage system is revised according to the manufacturer's recommendation.

The manufacturer's recommended installation methods shall be followed.

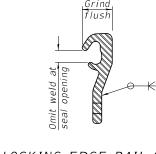
All steel components shall be galvanized after fabrication according to Article 520.03 of the Standard Specifications.

The Maximum space between locking edge rail segments shall be $\frac{3}{16}$ " and sealed with a suitable sealant; however, any rail joint within 10' measured perpendicular to the face of the curb or parapet shall be welded as shown in the locking edge rail splice detail.

The top surface of sidewalk sliding plates shall have a raised pattern according to ASTM A786.

Cost of parapet sliding plates, sidewalk sliding plates, embedded plates, anchorage studs, and expansion anchors included with Preformed Joint Strip Seal.

34" F-shape barrier shown, 42" F-shape similar as noted. The concrete opening below the strip seal will vary based on the locking edge rail chosen by the Contractor. Deck and parapet lengths shown elsewhere in the plans are dimensioned to the concrete opening, not the joint opening, and are based on the rolled locking edge rail. If the Contractor elects to use a different locking edge rail, dimensional adjustments may be required. One exception to this would be the strip seal joint at the end of the precast bridge approach slab. For these cases the pavement connector length shall be adjusted, not the length of the bridge approach slab.



LOCKING EDGE RAIL SPLICE

The inside of the locking edge rail groove shall be free of weld residue. Rolled rail shown, welded rail similar.

BILL OF MATERIAL

Item	Unit	Total
Preformed Joint Strip Seal	Foot	80

EJ-SS-S 8-11-17

| DESIGNED - ___ REVISED - | DRAWN - ____ REVISED - | DRAWN - ____ REVISED - | DRAWN - ____ REVISED -



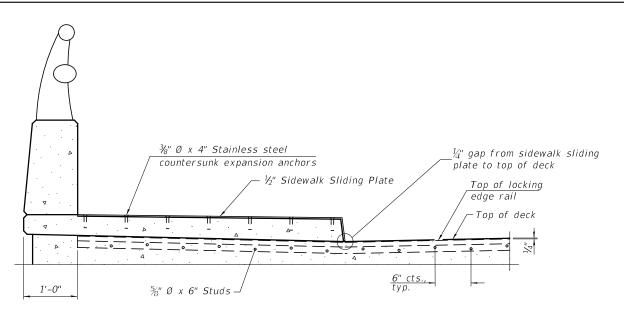
Locking edge rail-

Top of concrete

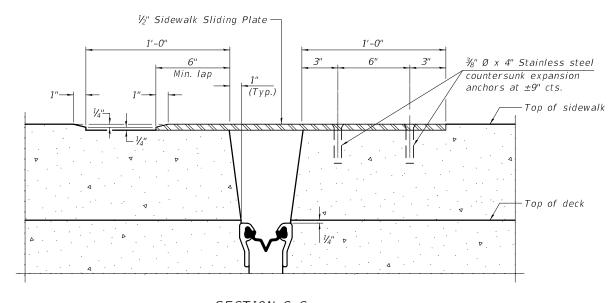
DEPARTMENT OF ENGINEERING DANVILLE, IL R. DAVID SCHNELLE, CITY ENGINEER Scale: N.A. = 1'-0" OREGON ST BRIDGE REPAIRS PREFORMED JOINT SEAL-SIDEWALK

	RTE.	PROJECT NUMBER		COUNTY	TOTAL SHEETS	SHEET NO.
		16-00352-00-BR		VERMILION	13	09
*OREGON			SHT. S-09	OF S	-13	
	2017	BRIDGE REPAIRS	ST	RUCTURE N	0.092	-6017

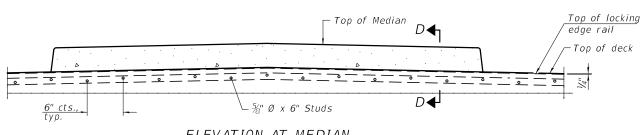
(Sheet 1 of 3)



ELEVATION AT RAISED SIDEWALK

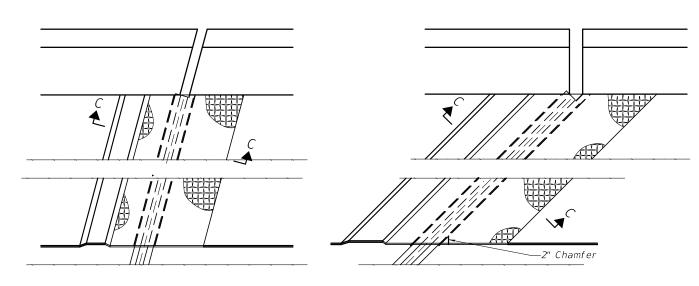


SECTION C-C



ELEVATION AT MEDIAN

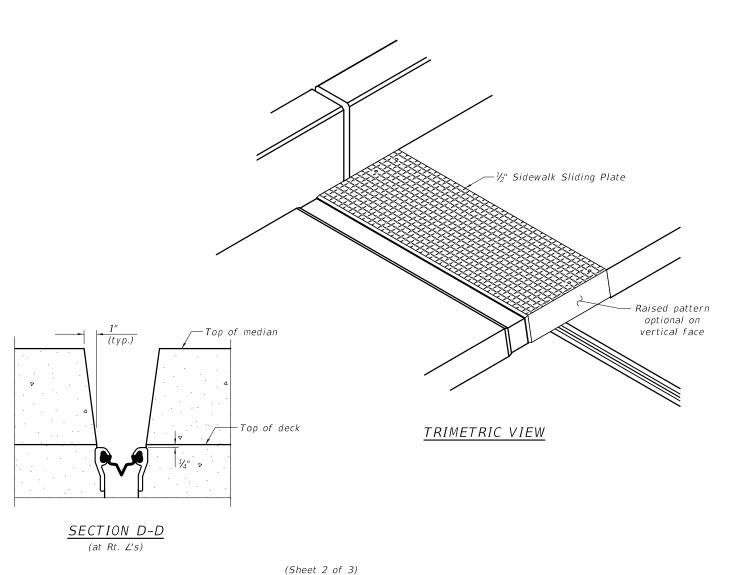
For skews > 30°, chamfer acute corners 2" similar to sidewalk.



 $(FOR SKEWS \le 30^{\circ})$

(FOR SKEWS > 30°)

PLAN AT RAISED SIDEWALK



EJ-SS-S 8-11-17

DEPARTMENT OF ENGINEERING

OREGON ST BRIDGE REPAIRS PREFORMED JOINT SEAL-SIDEWALK

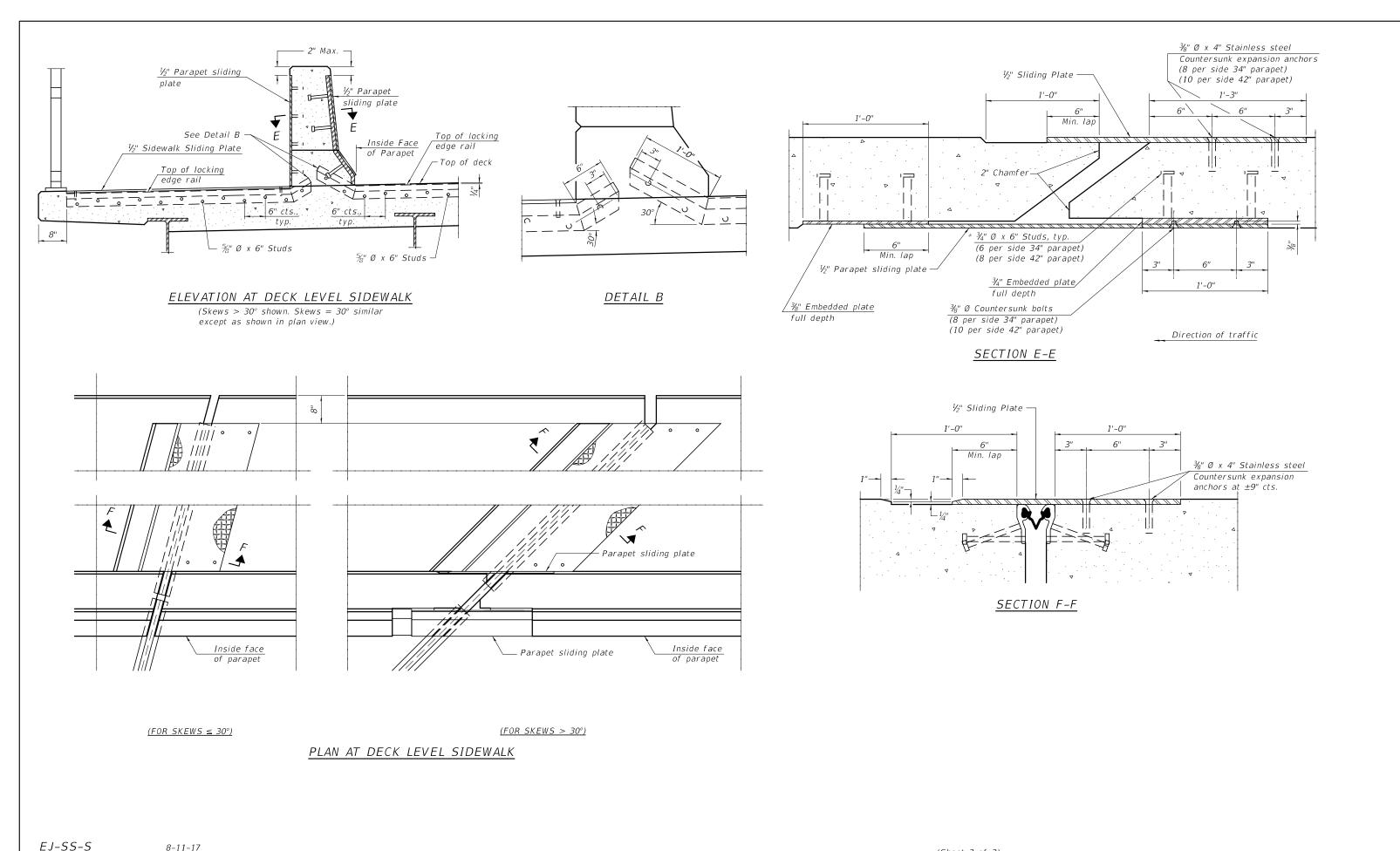
COUNTY TOTAL SHEET NO.

VERMILION 13 10 PROJECT NUMBER • 16-00352-00-BR SHT. S-10 OF S-13

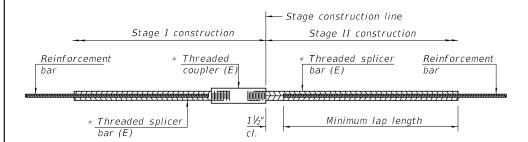
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DESIGNED -REVISED DRAWN REVISED CHECKED REVISED - ---- 1/3/2018 DATE REVISED

DANVILLE, IL R. DAVID SCHNELLE, CITY ENGINEER Scale: N.A. = 1'-0''



8-11-17 (Sheet 3 of 3) FILE LOCATION = X:/PROJECTS/CITY/CURREN DESIGNED -REVISED PROJECT NUMBER COUNTY OREGON ST BRIDGE REPAIRS DEPARTMENT OF ENGINEERING DRAWN REVISED • 16-00352-00-BR VERMILION 13 11 PREFORMED JOINT SEAL-SIDEWALK CHECKED REVISED DANVILLE, IL R. DAVID SCHNELLE, CITY ENGINEER - ---- 1/3/2018 SHT. S-11 OF S-13 DATE REVISED Scale: N.A. = 1'-0"

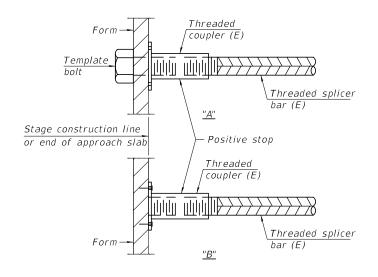


STANDARD BAR SPLICER ASSEMBLY

Threaded splicer bar length = min. lap length + $1\frac{1}{2}$ " + thread length

* Epoxy not required on Bar Splicer Assembly components used in conjunction with black bars.

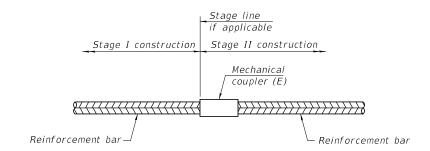
Location	Bar	No. assemblies	Minimum
Location	size	required	lap length
Slab	#5	16	3′-5"
Approach Pavements	#4	12	2'-2"
Approach Pavements	#5	16	3'-0"
Approach Pavements	#6	48	3'-2"
Abutments	#6	8	3′-6"



INSTALLATION AND SETTING METHODS

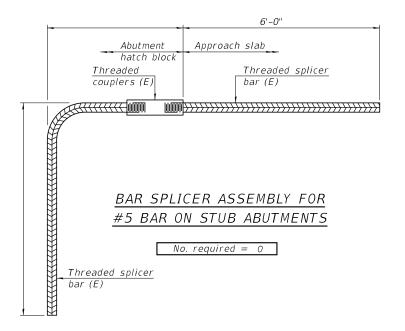
"A" : Set bar splicer assembly by means of a template bolt. "B" : Set bar splicer assembly by nailing to wood forms or cementing to steel forms.

(E): Indicates epoxy coating.



STANDARD MECHANICAL SPLICER

Location	Bar size	No. assemblies required



NOTES

Splicer bars shall be deformed with threaded ends and have a minimum 60 ksi yield strength.

All reinforcement shall be lapped and tied to the splicer bars.

Bar splicer assemblies shall be epoxy coated according to the requirements for reinforcement bars. See Section 508 of the Standard Specifications. See approved list of bar splicer assemblies and mechanical splicers for alternatives.

BSD-1

2-17-2017

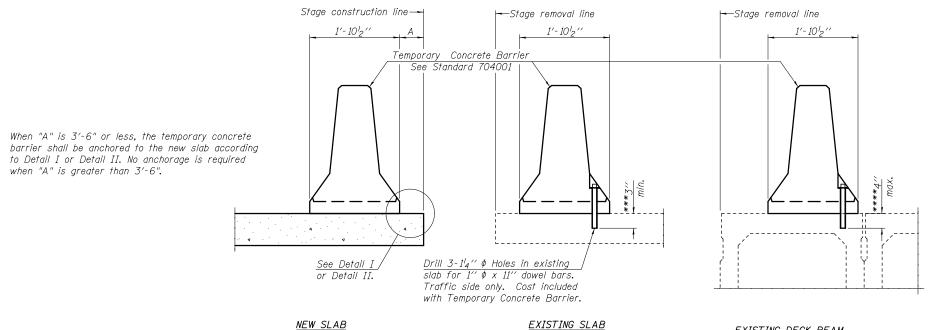
FILE LOCATION = X:/PROJECTS/CITY/CURREN DESIGNED -REVISED DRAWN REVISED CHECKED REVISED DATE - 1/3/2018 REVISED



DEPARTMENT OF ENGINEERING DANVILLE, IL R. DAVID SCHNELLE, CITY ENGINEER Scale: N.A. = 1'-0"

BAR SPLICER ASSEMBLY AND MECHANICAL SPLICER DETAILS STRUCTURE NO. Ø92-6Ø17

PROJECT NUMBER RTE. COUNTY • 16-00352-00-BR VERMILION 13 12



NOTES

Detail I - With Bar Splicer or Couplers: Connect one (1) 1" x 7" 'x "W" steel P to the top layer of couplers with 2-58" \$\phi\$ bolts screwed to coupler at approximate € of each barrier panel.

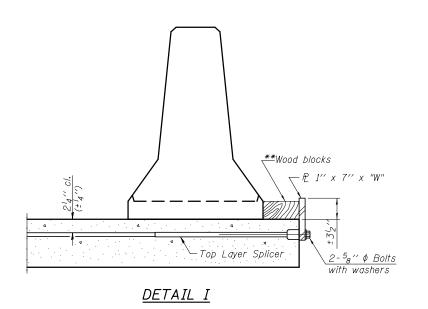
Detail II - With Extended Reinforcement Bars: Connect one (1) $1'' \times 7'' \times 'W''$ steel P to the concrete slab or concrete wearing surface with 2- $\frac{5}{8}$ " ϕ Expansion Anchors or cast in place inserts spaced between the top layer of reinforcement at approximate © of each barrier panel.

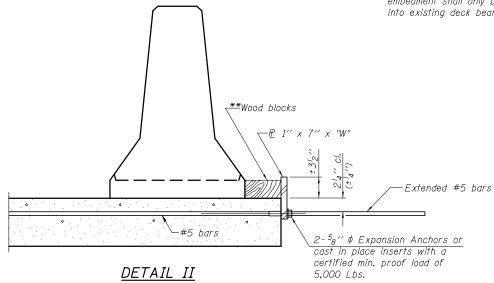
Cost of anchorage is included with Temporary Concrete Barrier. The 1" x 7" x "W" plate shall not be removed until stage II construction forms and all reinforcement bars are in place and the concrete is ready to be placed.

SECTIONS THRU SLAB OR DECK BEAM

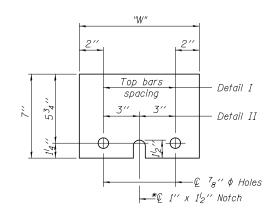
EXISTING SLAB

- *** Dimension shown is minimum required embedment into concrete. If hot-mix asphalt wearing surface is present, minimum embedment shall be in addition to wearing surface depth.
- **** If existing deck beam is to remain in place after stage construction, embedment shall only be into wearing surface and not into existing deck beam concrete.





EXISTING DECK BEAM



STEEL RETAINER P 1" x 7" x "W"

* Required only with Detail II

** Wood blocks may be omitted when required to provide minimum stage traffic lane width. When the wood blocks are omitted, the concrete barrier shall be in direct contact with the steel retainer plate.

"W" = Top bars spacing + 4"

R-27

FILE LOCATION =

7-1-10

DESIGNED	-		REVISED	-
DRAWN	-		REVISED	-
CHECKED	-		REVISED	-
DATE	-	1/3/2018	REVISED	-



DEPARTMENT OF ENGINEERING DANVILLE, IL R. DAVID SCHNELLE, CITY ENGINEER Scale: N.A. = 1'-0"

TEMPORARY CONCRETE BARRIER FOR STAGE CONSTRUCTION STRUCTURE NO. Ø92-6Ø17

PROJECT NUMBER COUNTY • 16-00352-00-BR VERMILION 13 13 *OREGON 2017 BRIDGE REPAIRS SHT. S-13 OF S-13 STRUCTURE NO. 092-601